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A. Appendix A

APPLICABILITY OF RBSM METHOD IN SMALL MESH SPECIMENS

A.1. Introduction

In this study, specimen mesh sizes are relatively small. That is 5-10 mm meshes in the concrete cover thickness and 15-40 mm meshes on the other areas of the specimen. The relatively small meshes enable to observe crack propagation precisely in the concrete cover.

The applicability of the RBSM analytical method for small mesh size is calibrated and the proposed modifiers for the concrete properties parameters are given for the analysis as shown in **Table A.1**. The calibration of the RBSM model is conducted for several testes and the analytical results are verified against the test results. **Table A.1** shows parameter modifiers using in the analysis. In which, parameters for tensile behavior of normal springs are modified since the tensile behavior strongly depends on contact surface area of rigid body. So, the smaller mesh size will result larger contact surface area. Parameters for compressive behavior of normal springs and for shear behavior of shear springs are not modified.

Parameter	Modifier	Analytical value	
G_F (N/mm)	0.505	0.505 *G _F	
f_t (MPa)	0.850	0.850 *f _t	
f' _c (MPa)	1.000	1.000 *f _c	
E_c (MPa)	1.000	1.000 *E _c	

Table A.1. Parameter modifiers in the analysis

A.2. "Direct" tension test

The experiment was carried out by Cornelissen et al. (1986) to test a concrete specimen with double notches as shown in **Figure A**.1. The purpose to calibrate the analytical model in this specimen is that the tension behavior of concrete is dominant in the case of corrosion induced expansion (Lundgren, 2002; Nguyen et al., 2006). This calibration is to verify the proposed modified parameters as mentioned above.

The specimen was simulated with the 5mm meshes around the notches and 15 mm in the other areas as shown in **Figure A.2**. The concrete properties parameters used in the analysis are determined from the parameter modifiers in **Table A.1**. The analytical results including the relationship of average displacement of the 35mm- gauges and the tension stress σ (MPa) and the 3D deformation of the specimen were compared with the test results as shown in **Figure A**.3. The analytical results appear reasonable agreement for tensile strength and post peak behavior Crack patterns are also understood from the deformation, because the deformation is occurred by the separation between rigid bodies in RBSM. The reasonable crack patterns and propagation behavior are simulated.



Figure A.1. Outline of "direct" tension test



Figure A.2. RBSM modeling of "direct" tension specimen



Deformation after peak regime (magnification = 25)

Figure A.3. Analytical results of "direct" tension specimen

A.3. Pre-cracked three-point bending test

In order to calibrate the RBSM analytical model in the bending test, pre-cracked three-point bending beams with a 3 mm wide pre-crack are loaded. The set-up of the beam is shown in the **Figure A.4**. The purpose to calibrate the analytical model in this

specimen is to verify the crack propagation under the bending behavior of concrete because a specimen tends to be bent in the case of rebar corrosion induced expansion (Nguyen et al., 2006). This calibration is to verify the proposed modified parameters as mentioned above.

The specimen was simulated with 5 mm meshes around the pre-crack and 10 mm in the other areas as shown in **Figure A.5**. The parameters used in the analysis are shown in **Figure A.4**. The relationship between the load and the CMOD (Crack Mouth Opening Displacement) obtained from the analysis is compared with the test results in **Figure A.6**. The 3D deformation of the beam showing crack patterns at small and large CMOD values is also shown in **Figure A.6**. The analytical results appear well agreement with the test results in both pre-peak and post-peak regimes. Especially, the applicability to large CMOD value, which is larger than 1 mm, is confirmed.



Figure A.4. Set-up of pre-cracked three point bending specimen



Figure A.5. Meshing of pre-cracked 3- point bending test



Deformation @CMOD = 1.00 mm Magnification =3

Figure A.6. Analytical results of pre-cracked three point bending test

A.4. Cylinder compression test

Although compressive behavior does not directly occur in the case of corrosion induced expansion specimen, the shear behavior can be verified in the compression test since the shear behavior is dominant in meso-level for compression failure. This validation is to verify the assumed shear parameters as mentioned in the concrete model.

Watanabe and Niwa (2004) carried out a compression test for a $\phi 100*200$ mm cylinder concrete with concrete properties used in the analysis as shown in **Figure A.7**. The specimen is modeled with a unique 10 mm mesh size as shown **Figure A.8**. The stress-strain relationship obtained from the analysis is compared with the test results as shown in **Figure A.9**. Again, the analytical results reasonably agree with the test results.

The above verifications against the various specimen tests show the applicability of the RBSM analytical model in concrete structural analysis for small mesh size less than 10 mm.



	Parameter	Experimental value	Analytical value
	G _f (N/mm)	0.078	0.039
200	f _t (MPa)	2.28	1.933
	f' _c (MPa)	31.10	31.10
	E _c (MPa)	2.62E+04	2.62E+4

Figure A.7. Set-up of cylinder compression test



Figure A.8. RSBM modeling of compression cylinder



Figure A.9. Analytical results of cylinder compression specimen

B. Appendix B

EXPERIMENTAL RESULTS

B.1. Introduction

In appendix B, the experimental results of the specimen series conducted in this study are presented.

B.2. Specimen series 150*150-C30





Figure B.1. Surface crack width propagation of 150*150-C30

B.2.2. Surface crack pattern

Corrosion amount: 445 mg/cm² Surface crack width: 0.94 mm, 0.93 mm

Figure B.2. Typical notation on crack patterns



Figure B.3. Surface crack patterns of 150*150-C30



Figure B.4. Internal crack patterns of 150*150-C30

B.2.4. Actual rebar corroded sectional pattern



Figure B.5. Actual rebar corroded sectional pattern of 150*150-C30

B.3. Specimen series 150*150-C7.5

B.3.1. Internal crack pattern



Figure B.6. Internal crack pattern of 150*150-C7.5

B.3.2. Actual rebar corroded sectional pattern



Figure B.7. Actual rebar corroded sectional pattern of 150*150-C7.5

B.4. Specimen series 150*150-C15



Figure B.8. Surface crack width propagation of 150*150-C15

B.4.2. Surface crack pattern



Figure B.9. Surface crack patterns of 150*150-C15

B.4.3. Internal crack pattern



Figure B.10. Internal crack patterns of 150*150-C15

B.4.4. Actual rebar corroded sectional pattern



Figure B.11. Actual rebar corroded sectional pattern of 150*150-C15

B.5. Specimen series 150*150-C45

B.5.1. Surface crack propagation



Figure B.12. Surface crack width propagation of 150*150-C45

B.5.2. Surface crack pattern



Figure B.13. Surface crack patterns of 150*150-C45

B.5.3. Internal crack pattern



Figure B.14. Internal crack patterns of 150*150-C45

B.5.4. Actual rebar corroded sectional pattern



Figure B.15. Actual rebar corroded sectional pattern of 150*150-C45

B.6. Specimen series 150*150-D10



B.6.1. Surface crack propagation

Figure B.16. Surface crack width propagation of 150*150-D10

B.6.2. Surface crack pattern



Figure B.17. Surface crack patterns of 150*150-D10

Internal crack pattern *B.6.3*.



Figure B.18. Internal crack patterns of 150*150-D10

0.64mm

B.6.4. Actual rebar corroded sectional pattern



Figure B.19. Actual rebar corroded sectional pattern of 150*150-D10

B.7. Specimen series 150*150-D25



B.7.1. Surface crack propagation



B.7.2. Surface crack pattern



Figure B.21. Surface crack patterns of 150*150-D25

B.7.3. Internal crack pattern



Figure B.22. Internal crack patterns of 150*150-D25

B.7.4. Actual rebar corroded sectional pattern



Figure B.23. Actual rebar corroded sectional pattern of 150*150-D25

B.8. Specimen series 300*150-C30

B.8.1. Surface crack propagation



Figure B.24. Surface crack width propagation of 300*150-C25

B.8.2. Surface crack pattern



Figure B.25. Surface crack patterns of 300*150-C30

B.8.3. Internal crack pattern



Figure B.26. Internal crack patterns of 300*150-C30

B.8.4. Actual rebar corroded sectional pattern



Figure B.27. Actual rebar corroded sectional pattern of 300*150-C30

B.9. Specimen series 250*250-C30



Figure B.28. Surface crack width propagation of 250*250-C30



B.9.2. Surface crack pattern

Figure B.29. Surface crack pattern of 250*250-C30

B.9.3. Internal crack pattern



Figure B.30. Internal crack pattern of 250*250-C30

B.9.4. Actual rebar corroded sectional pattern



Figure B.31. Actual rebar corroded sectional pattern of 250*250-C30

B.10. Specimen series 400*400-C30





Figure B.32. Surface crack width propagation of 400*400-C30





Figure B.33. Surface crack pattern of 400*400-C30

B.10.3. Internal crack pattern



Figure B.34. Internal crack pattern of 400*400-C30

B.10.4. Actual rebar corroded sectional pattern



Figure B.35. Actual rebar corroded sectional pattern of 400*400-C30

B.11. Specimen series 600*400-C30

B.11.1. Surface crack propagation



Figure B.36. Surface crack width propagation of 600*400-C30

B.11.2. Surface crack pattern



Figure B.37. Surface crack pattern of 600*400-C30

B.11.3. Internal crack pattern



Figure B.38. Internal crack pattern of 600*400-C30

B.11.4. Actual rebar corroded sectional pattern



Figure B.39. Actual rebar corroded sectional pattern of 600*400-C30